







# AN ASSESSMENT OF THE FAILURE OF THE WADI DERNA DAMS AND LESSONS FOR ENHANCING DAM SAFETY



Laurent Mouvet
Civil Eng. EPFL-SIA
Vice President ICOLD
Past President SwissCOLD













#### Joint Case Analysis

- > ICOLD International Commission on Large Dams
- Netherland's DRRS (Dutch Risk Reduction & Surge Programme)
- UNESCO
- WORLD BANK

Workshop with the Libyan authorities on 15-16 April

Final Report to be released very soon

A Conference edition was released for the ICOLD Annual Meeting 2024 in New Delhi (not for citation or publication)











### **BREAKING POINT**

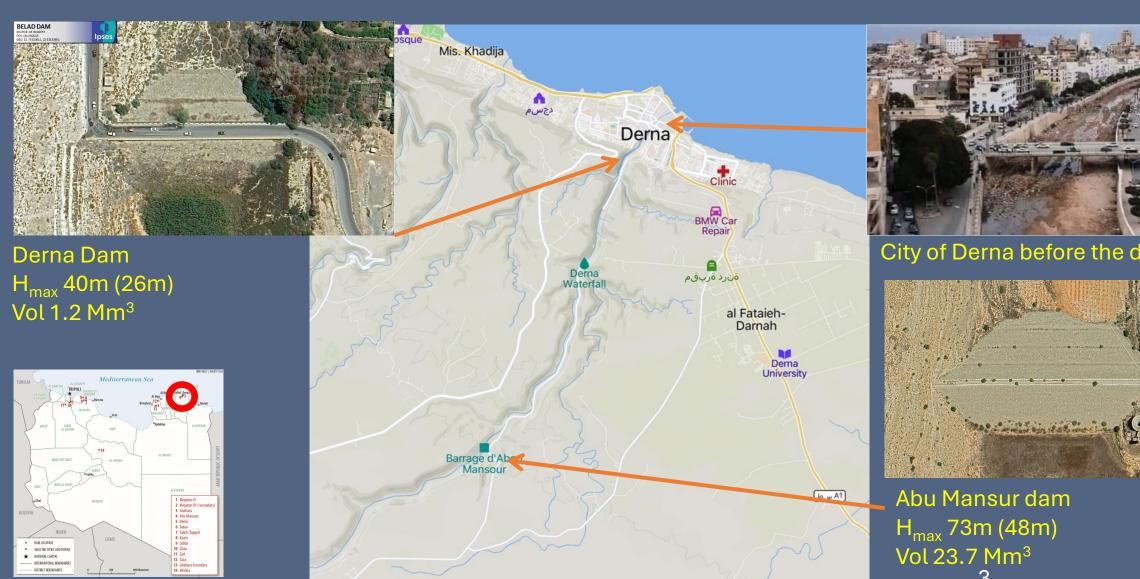
AN ASSESSMENT OF THE FAILURE OF THE WADI DERNA DAMS AND LESSONS FOR ENHANCING DAM SAFETY

























## **Storm Daniel**

- Made landfall in Libya around September 9th
  - Storm Daniel's west-east path is the worst-case scenario for the accumulation of runoff in Wadi Derna, resulting in high rainfall quantities with an intensity that were much larger than the infiltration capacity.
- Rainfall event of circa 150-300 mm
  - 414 mm rainfall reported by National Meteorological Agency
  - Satellite-based precipitation measurements show consistently lower rainfall intensity
  - Optical satellite imagery was also used to identify several waterbodies that filled during the storm event and identify precipitation levels between 120-200 mm.
  - The return period of the event is estimated between 500 and 1000 years









## Magnitude of the flood

- The design flow for the dams was based on a rainfall event with a return period of 1000 years.
- The estimates of the return period associated with the peak flow and flood volume of the event are **considerably higher** than the original design estimates.
  - The 2 dams were dramatically under-designed from the hydrological point of view
  - Spillway capacity and reservoir volume out of proportion compared to the Sept. 2023 flood

#### Hydraulic design criteria related to flood protection for Derna and Abu Mansur Dams

Dam	Derna			Abu Mansur		
	Original design (1972)	Stucky Review (2003)	Model Estimates (2023)	Original design (1972)	Stucky Review (2003)	Model Estimates (2023)
Return period (year)	1,000	1,000	1,000	1,000	1,000	1,000
Peak flow (m <sup>3</sup> /s)	~350	906	1,750	840	1,360	1,950
Flood volume (Mm³)	4	35.4	61	14	47.6	78
Maximum released flow (m³/s)	350	570		170	420	

Vol reservoir 1.2 Mm<sup>3</sup>

Vol reservoir 23.7 Mm<sup>3</sup>



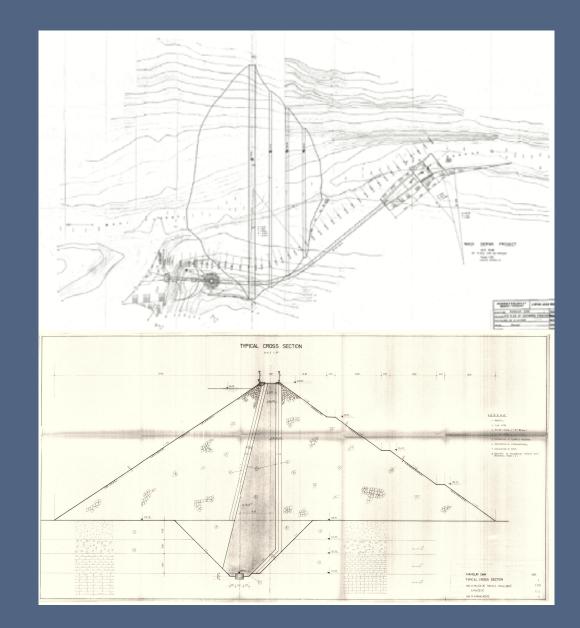






## Abu Mansur Dam

- Embankment dam with a thin clay core
- ➤ Located ~11 km upstream of Derna Dam
- Catchment area = 476 km²
- ➤ Height = 73 m from the foundation
- ➤ Storage Volume = 23.7 MCM
- Morning-glory type overflow spillway
- > Spillway capacity = 170 m<sup>3</sup>/s











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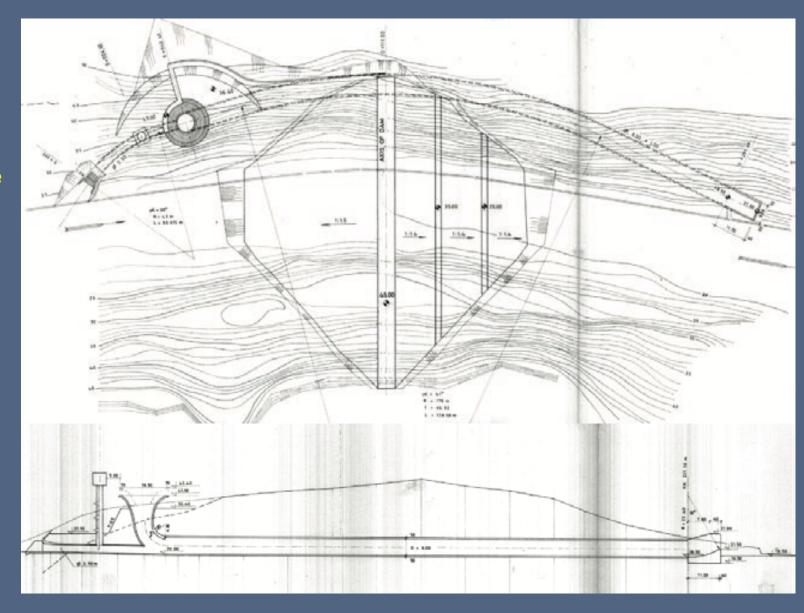






## Derna Dam

- Embankment dam with a thin clay core
- Located ~1 km upstream of Derna city
- Catchment area = 575 km²
- ➤ Height = 40 m from the foundation
- Storage Volume = 1.15 MCM
- Morning-glory type overflow spillway
- > Spillway capacity = 350 m<sup>3</sup>/s











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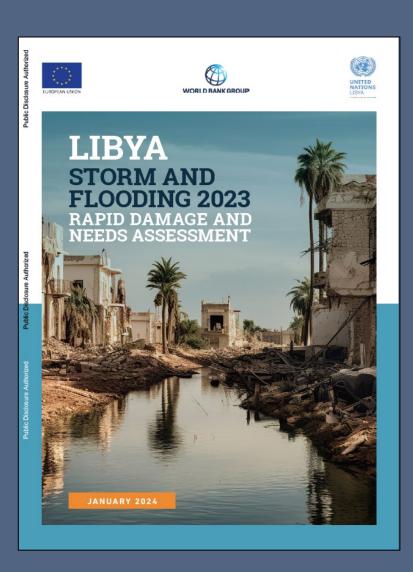












## **Storm Daniel – Damage Estimates**

- ➤ Damages and losses assessed at US\$1.65b, equivalent to 3.6% of Libya's GDP
- More than 250,000 people were affected, with fatalities exceeding 13,000 people, more than 8,000 people unaccounted for and 45,000 people displaced
- More than 18,500 houses are estimated to have been destroyed or damaged, equivalent to seven percent of the country's housing stock
- ➤ Initial reconstruction estimates in excess of US\$1.8b, with >70% for infrastructure, the largest component being the housing sector



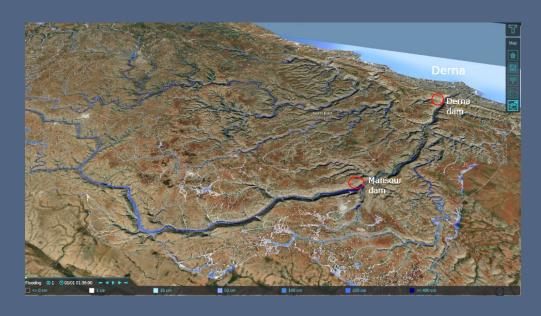






## Magnitude of the flood

- Storm Daniel's west-east path represents the worst-case scenario for the accumulation of runoff in Wadi Derna, resulting in high rainfall quantities with an intensity that were much larger than the infiltration capacity.
- A hydrological model was calibrated to several historic flood events of with a sensitivity analysis testing a rainfall panel ranging from 150 to 300 mm.













## A Wall of Water

The flood wave resulting from the failure

- The failure of the Derna Dam is estimated to have generated a first flood wave with a flow ranging between 1,500 and 5,000 m<sup>3</sup>/s.
- Simulations of the second flood wave that
  was caused by the collapse of Abu Mansour
  dam suggests an estimation of discharge of
  around 7,000 m³/s, significantly exceeding
  the capacity of the city's river channel, which
  could handle a maximum of around 1,000
  m³/s



The water level in Derna during the 2023 flood event.





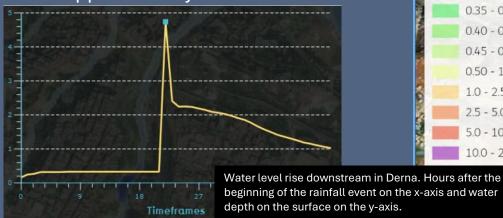


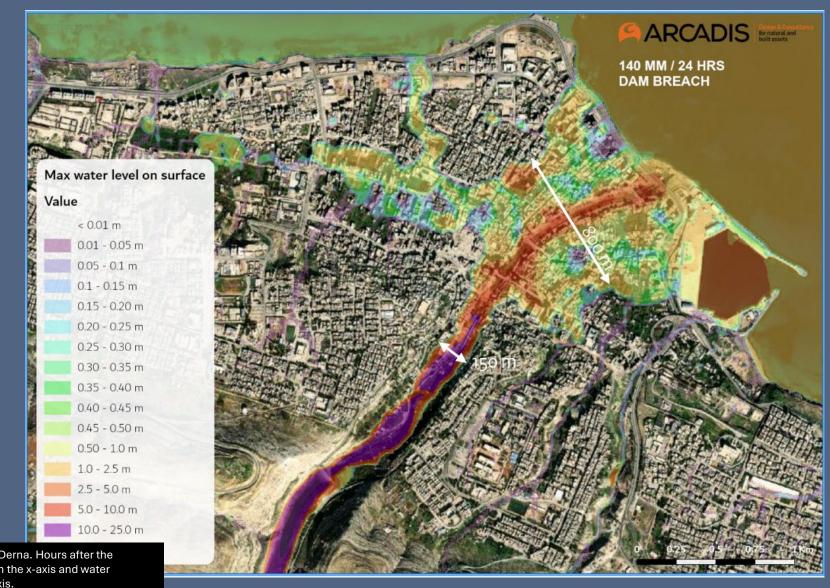


## Hydrological Model

Results of the max water level on surface after the dam breach

- The width of the main flow channel would have extended over 800 meters (indicated by white arrow)
- Water level rise in the city of Derna between approx. 5-8 masl within 30 minutes.
- The water level drops and sinks out at about 1 meter in approximately 24 hours.













## Chronology of dam safety reviews

#### Derna dam

H<sub>max</sub> 40m (26m) V=1.2 Mm<sup>3</sup>



#### Abu Mansur dam

H<sub>max</sub> 73m (48m) V=23.7 Mm<sup>3</sup>



Design 1970-74

Commissioning: 1978

Monitoring report 1984, by the contractor

- excessive settlement of both dams
- watertightness issue at the two dams
   3 m³/s under less than 14m head at Abu Mansur,
   3 sinkholes at immediate US Derna

### Libyan dam inspection, UNDP 1994

- Concern with the settlement at Abu Mansur dam crest
- Recommendation: periodic inspections

Safety assessment and rehabilitation project 2004

- Reassessment of the hydrology
- Design of new spillway facilities
- Improvement of water tightness
- Upgrade the monitoring system

#### Work never started

Contract cancelled in 2010









## Crest Settlement - Abu Mansur Dam, 2004





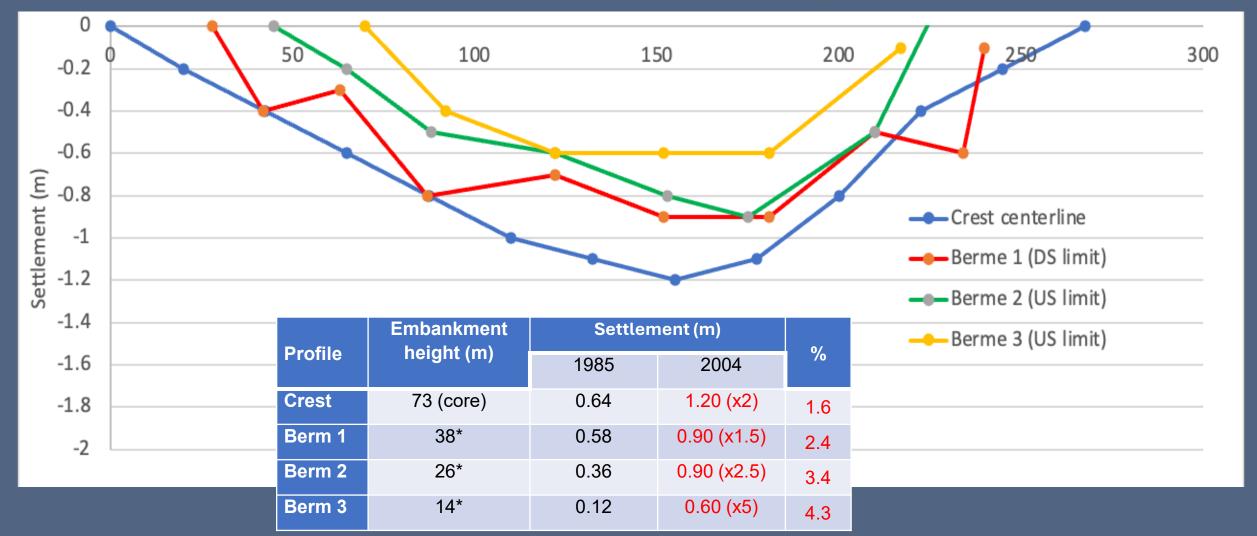


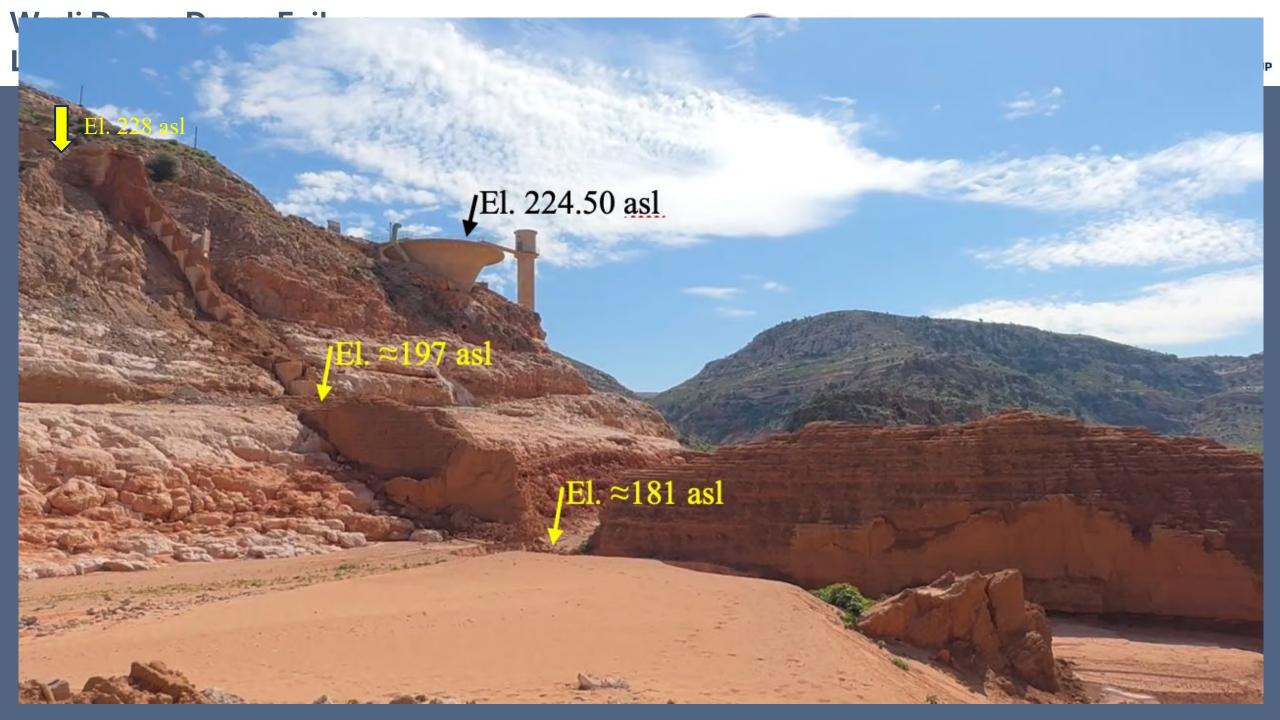






## Crest Settlement – Abu Mansur Dam, 2004





## Wadi Darna Dame Failura TYPICAL CROSS SECTION Z=228 masl \*\* H max: 73m H on the riverbed: 48/50m Hydraulic gradient at riverbed level: LEGEND: I=(226-179)/9=5.2 Foundation permeability: Alluvium maximum K=1x10<sup>-2</sup> cm/s thickness=25m K=1x10-1 cm/s K=1x10-4 cm/s 1:200 S. RAIČEVIĆ ING. R. ATANACKOVIC

LIMESTONE NUMMULITIE Wadi CONGLOMERATES, STRONGLY JOINED. BROKEN, VERY POROUS SOFT, VERY PORGUS Libya HARD SILICEOUS LIMESTONE GRIS, VERY HARD OBSERVATED LARGE OPEN CAVITIES GEDLOGICAL SECTION L=20m, Hmax=3 to 4m Left bank Right ban DESCRIPTION

H=344+

Figure 4-4: Bu Mansur dam: Geological cross section along the foundation of the core (Hidroprojekt, 1975/76)

MARKING MOREHOLES

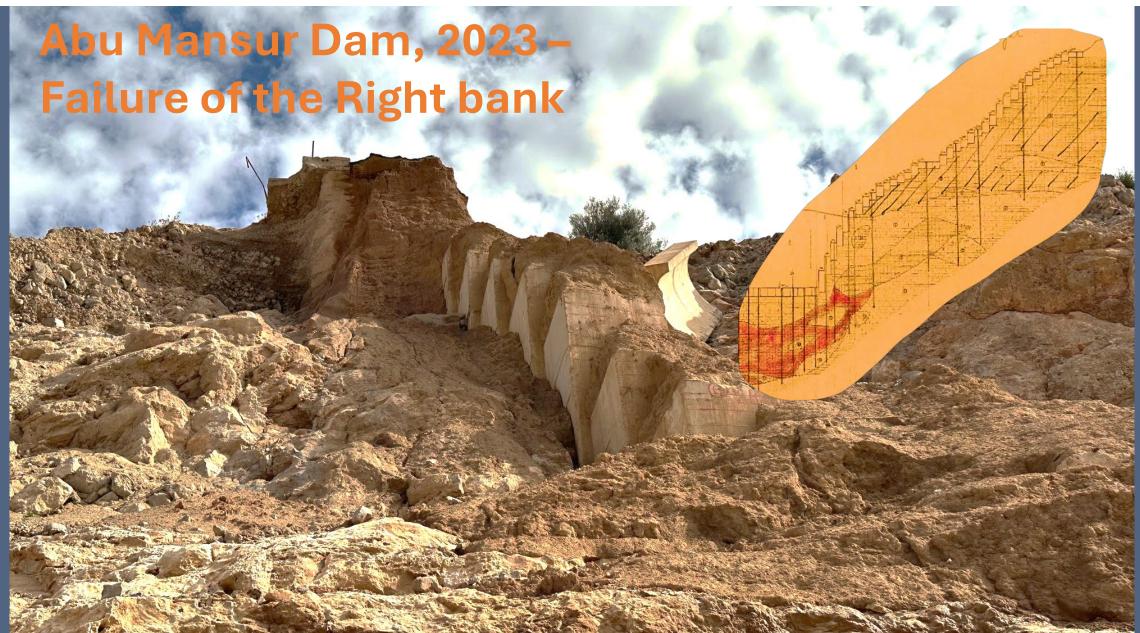
O , BOPPHOLES IST STAGE



















# Abu Mansur Dam, 2023 Spillway intake is intact

Did it spill on the 11<sup>th</sup> September 2023?











Abu Mansur Dam, 2023
Failure of the Left bank

What is the sliding plan on the left bank?











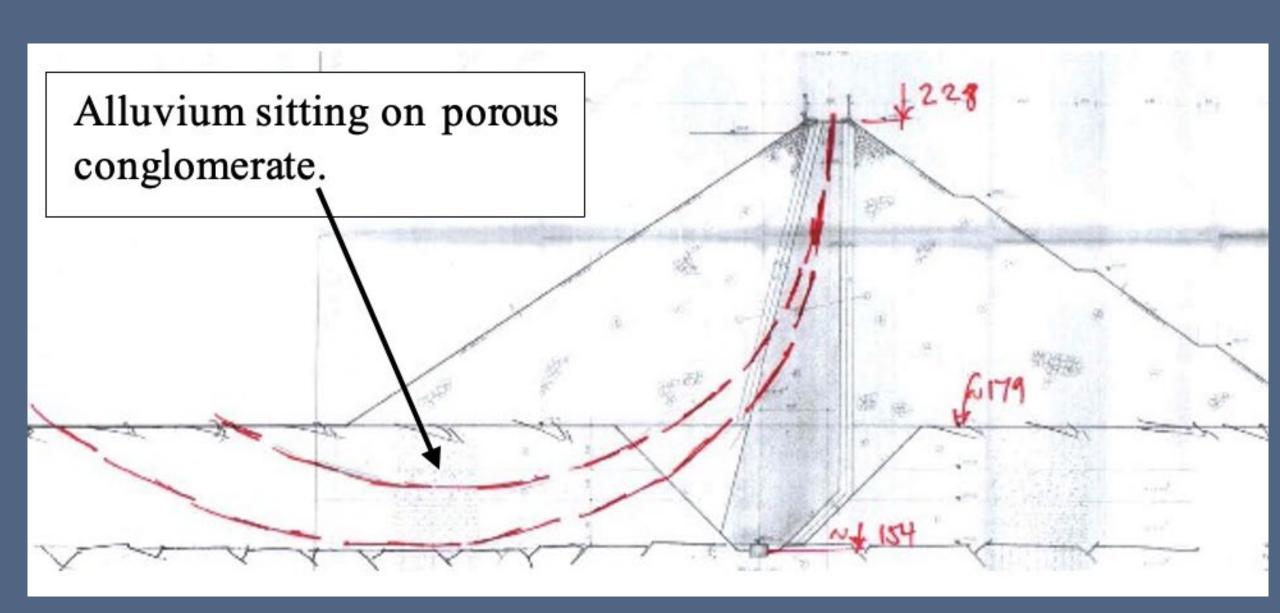








































## Lessons learned

- Unclear Purpose and Protection Concept
- Design Criteria
- Periodic Safety Assessment and Surveillance
- Mitigation Measures to be implemented
- Risk Assessment and Emergency Preparedness Plan
- Flood forecasting
- Warning System

- Inadequate Design
- Institutional and Organisational failure









## Purpose and Protection concept

According to the design documentation

- > Flood protection for the city of Derna
- > Aquifer recharge
- > Fataya Agriculture Project





Derna City Extension in 1943



Derna City Extension in 2022









## Design Criteria

#### **Project specifications**

#### Type of dam

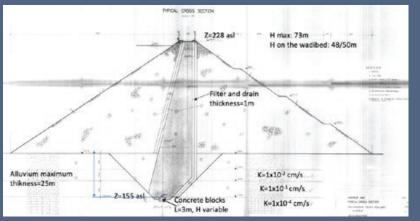
 Earthfill embankment with clay core

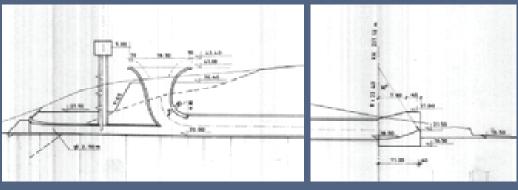
#### Type of Spillway

Morning glory spillways

Is the Project resilient?

- Limited knowledge of the hydrology of the watershed at the design phase
- > Level of Protection not clearly defined
- Reservoir is empty most of the time
- Rapid fillings
- No possibility of dam monitoring during the fillings
- Limited control of seepage
- High uncertainties in the assessment of the flood volume and peak discharge
- > Free flow spillway
- Limited capacity













## Safety Assessment and Surveillance

#### Structural Safety

- X Design as per standards and practice
- ? Construction according to specifications
- X Periodic review of safety conditions (5-10 years):

Better knowledge of local conditions

Better knowledge of the dam behavior

Progress of science and practice

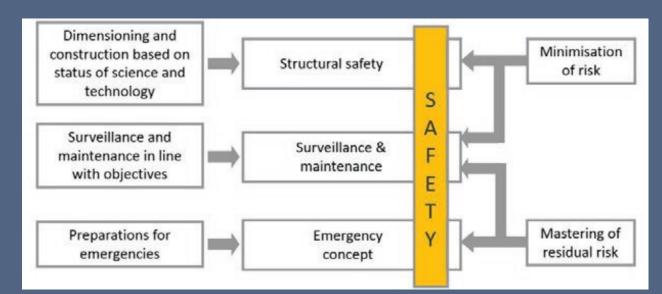
Change in regulations and social expectations

#### Surveillance and Maintenance

- X Maintain the dam in the condition it was designed for
- X Maintain the dam safety outlets in operational conditions, tests
- ? Confirm that the dam is behaving as expected during the design phase
- ? Identify abnormal behavior

#### Emergency Concept

- X Prepare the protection of population, goods, and infrastructures in case of unexpected release of water
- X Develop a flood forecasting system
- X Install warning systems if necessary
- X Define and train the emergency warning system and communication chain





Predictable accident with major damages









## Post-Disaster Actions

#### **DEFINE NATIONAL AND SUB-NATIONAL PRIORITIES**

- 1. Carry out a more detailed assessment of the 2023 flood event
- 2. Carry out a more detailed assessment of the dam failures
- 3. Improve flood forecasting, early warning systems and emergency preparedness
- 4. Take into account the climate change impact on flood prediction
- 5. Determine the level of protection for the city of Derna
- 6. Determine appropriate interventions for building back better
- 7. Develop a regulatory framework for dam safety assurance
- 8. Launch a national dam safety program
- 9. Implement a stakeholder engagement and communication plan











## Thank you for attention



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https://kdrive.infomaniak.com/app/share/595022/185ad19a-2e6d-4440-bad3-8bc239737763